

## 5.2.4 Shear and Bending Interaction

### 5.2.4.1 Method

The design web shear capacity determined in Section 5.2.2.4 may be significantly reduced when the section is subject to a large design bending moment at the same location. The reduced design shear capacity ( $\phi V_{vm}$ ) is determined in accordance with Clause 5.12.3 of AS 4100 as:

$$\begin{aligned}\phi V_{vm} &= \phi V_v && \text{for } M^* \leq 0.75\phi M_s \\ \text{or} &= \phi V_v \left[ 2.2 - \left( \frac{1.6M^*}{\phi M_s} \right) \right] && \text{for } 0.75\phi M_s < M^* \leq \phi M_s\end{aligned}$$

where  $\phi V_v$  = design web shear capacity (see Sections 5.2.1 and 5.2.2.4)

$M^*$  = design bending moment

$\phi M_s$  = design section moment capacity (see Sections 5.2.1 and 5.2.2.1)

Designers must ensure that  $V^* \leq \phi V_{vm}$ .

**Note:** If  $V^* \leq 0.6\phi V_v$  or if  $M^* < 0.75\phi M_s$  then no check on the interaction of shear and bending is necessary.

### 5.2.4.2 Example

An example of a check on shear and bending interaction is given in Section 5.3.6.

## 5.2.5 Bending and Bearing Interaction

### 5.2.5.1 Method

The design web bearing capacity determined in Section 5.2.2.5 of the Tables may be significantly reduced when the section is subject to a large bending moment at the same location. The effect of this interaction of bending and bearing force in RHS and SHS is considered in AS 4100.

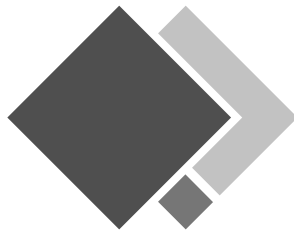
The bending and bearing interaction is dependent on the ratio of bearing length to the width of bearing ( $b_s/b$ ) and web slenderness ( $d_1/t$ ). Clause 5.13.5 of AS 4100 considers the following interaction to apply to RHS and SHS:

$$\begin{aligned}1.2 \left( \frac{R^*}{\phi R_b} \right) + \left( \frac{M^*}{\phi M_s} \right) &\leq 1.5 && \text{for } \frac{b_s}{b} \geq 1.0 \text{ and } \frac{d_1}{t} \leq 30 \\ \text{or} &0.8 \left( \frac{R^*}{\phi R_b} \right) + \left( \frac{M^*}{\phi M_s} \right) &\leq 1.0 && \text{otherwise}\end{aligned}$$

where

- $b_s$  = stiff bearing length (see Figure 5.2)
- $b$  = width of section
- $d_1$  = clear depth between flanges
- $t$  = thickness of section
- $R^*$  = maximum design bearing force
- $\phi$  = capacity factor = 0.9 (Table 3.4 of AS 4100)
- $\phi R_b$  = design web bearing capacity (see Sections 5.2.1 and 5.2.2.5)
- $M^*$  = maximum design bending moment
- $\phi M_s$  = design section moment capacity (see Sections 5.2.1 and 5.2.2.1)

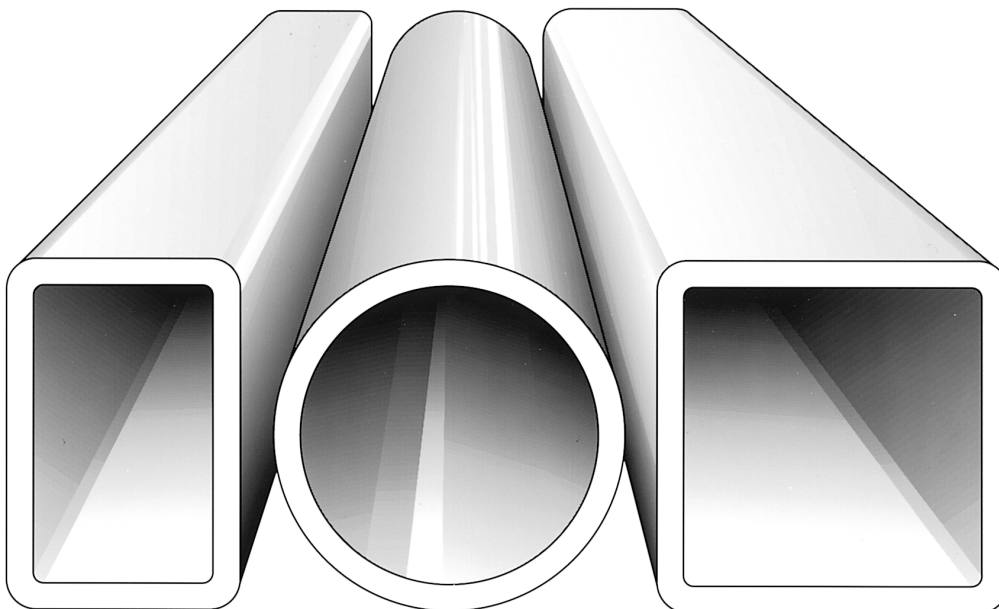
**Note:** These formulae only apply to bearing across the full width of section.



AUSTRALIAN STEEL INSTITUTE

(ABN) / ACN (94) 000 973 839

# design capacity tables for structural steel



## Volume 2: Hollow Sections

second edition

**CHS - Grade C250/C350 (to AS 1163)**

**RHS - Grade C350/C450 (to AS 1163)**

**SHS - Grade C350/C450 (to AS 1163)**

**LIMIT STATES  
EDITION TO  
AS 4100-1998  
 $S^* \leq \phi R_u$**

# design capacity tables for structural steel

## Volume 2: Hollow Sections

**second edition**

### TABLE OF CONTENTS

Foreword	(iv)
Acknowledgements	(iv)
Preface	(v)
Notation	(vi)

<b>PART ONE</b>	
Introduction	1-1

<b>PART TWO</b>	
Materials	2-1

<b>PART THREE</b>	
Section Properties	3-1

<b>PART FOUR</b>	
Methods of Structural Analysis	4-1

<b>PART FIVE</b>	
Members Subject to Bending	5-1

<b>PART SIX</b>	
Members Subject to Axial Compression	6-1

<b>PART SEVEN</b>	
Members subject to Axial Tension	7-1

<b>PART EIGHT</b>	
Members subject to Combined Actions	8-1

<b>PART NINE</b>	
Connections	9-1

INTRODUCTION

MATERIALS

SECTION  
PROPERTIES

METHODS OF  
STRUCT. ANAL.

MEMBERS  
SUBJECT  
TO BENDING

MEM. SUB.  
TO AXIAL  
COMPRESSION

MEM. SUB.  
TO AXIAL  
TENSION

MEM. SUB.  
TO COMBINED  
ACTIONS

CONNECTIONS

**PART 5****MEMBERS SUBJECT TO BENDING**

PAGE

<b>5.1</b>	<b>Maximum Design Loads for Beams with Full Lateral Restraint.....</b>	<b>5-3</b>
5.1.1	Strength Limit State Design .....	5-3
5.1.1.1	$W_{L1}^*$ based on Design Moment Capacity .....	5-3
5.1.1.2	$W_{L2}^*$ based on Design Shear Capacity .....	5-4
5.1.2	Serviceability Limit State Design .....	5-4
5.1.2.1	$W_S^*$ based on a Deflection Limit of $L/250$ .....	5-4
5.1.2.2	$W_{YL}^*$ based on First Yield Load.....	5-5
5.1.3	Full Lateral Restraint.....	5-5
5.1.4	Additional Design Checks.....	5-5
5.1.5	Other Load Conditions.....	5-5
5.1.6	Examples .....	5-7
<b>5.2</b>	<b>Design Section Moment and Web Capacities.....</b>	<b>5-9</b>
5.2.1	General.....	5-9
5.2.2	Method.....	5-9
5.2.2.1	Design Section Moment Capacity.....	5-9
5.2.2.2	Segment Length for Full Lateral Restraint (FLR).....	5-9
5.2.2.3	Design Torsional Moment Section Capacity.....	5-10
5.2.2.4	Design Shear Capacity of a Web .....	5-11
5.2.2.5	Design Web Bearing Capacities .....	5-11
5.2.3	Example – Web Bearing .....	5-13
5.2.4	Shear and Bending Interaction .....	5-15
5.2.4.1	Method .....	5-15
5.2.4.2	Example.....	5-15
5.2.5	Bending and Bearing Interaction.....	5-15
5.2.5.1	Method .....	5-15
5.2.5.2	Example.....	5-16
<b>5.3</b>	<b>Design Moment Capacities for Members Without Full Lateral Restraint .....</b>	<b>5-16</b>
5.3.1	General .....	5-16
5.3.2	Design Member Moment Capacity.....	5-17
5.3.3	Beam Effective Length .....	5-17
5.3.4	Other Loading and Restraint Conditions .....	5-17
5.3.5	Segment Length for Full Lateral Restraint.....	5-18
5.3.6	Examples .....	5-18

<b>5.4</b>	<b>Calculation of Beam Deflections .....</b>	<b>5-20</b>
<b>5.5</b>	<b>References .....</b>	<b>5-20</b>

## **TABLES**

### TABLES 5.1-1 to 5.1-6

Maximum Design Loads for Beams with Full Lateral Restraint .....	<b>5-22</b>
(A) Tables – Strength Limit State	
(B) Tables – Serviceability Limit State	

### TABLES 5.2-1 to 5.2-4

Design Section Moment and Web Capacities (RHS & SHS only) .....	<b>5-56</b>
(A) Tables – Bending about x-axis for RHS	
(B) Tables – Bending about y-axis for RHS	

### TABLES 5.3-1 to 5.3-2

Design Moment Capacities for Members without Full Lateral Restraint (RHS only) .....	<b>5-74</b>
---	-------------

<p><b>NOTE: SEE SECTION 2.1 FOR THE SPECIFIC MATERIAL STANDARD (AS 1163) REFERRED TO BY THE SECTION TYPE AND STEEL GRADE IN THESE TABLES</b></p>
--