#### Alternative paint systems

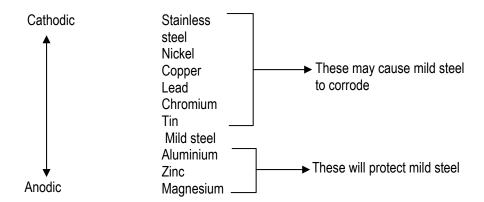
Available paints change as the paint industry develops new products, therefore the paint suppliers should be consulted before specifying a paint system for a particular project. For this reason, only a limited generic range of paints is mentioned in this section.

For non-exposed steel in a fully protected environment, painting is unnecessary and has been omitted on many such projects. However, steel in a protected environment is often specified to be painted for aesthetic reasons that include the avoidance of surface rust and the run-off of rusty water onto other surfaces during construction. An appropriate paint for these environments is zinc phosphate, with a dry film thickness of 50 or 75 microns. (Specify 50 microns for beams that are to have site welded shear studs. A thicker paint film may compromise the weld quality, and even with 50 microns, strict quality control will be necessary to detect faulty welds until it has been verified that defective welds are no more common than for an unpainted beam.)

For exposed steel in a non-marine environment, inorganic zinc silicate is an appropriate choice, either as the sole protective coating or as a priming coat for a multi-coat system. A dry film thickness (DFT) of 75 microns is normally considered to be the minimum thickness except where site welded shear studs are to be attached. In this case, the paint on the surface to have the studs attached (usually the top of the top flange of a beam) should be limited to a DFT of 50 microns. The DFT should always be regularly checked at the paint shop using a suitable DFT gauge and remedial measures should be taken when the thickness does not meet that specified. Paint must be applied before rust starts to discolour the cleaned surface. The steel must be neither too hot nor too cold and the surface must be at least 3 degrees above the dew point to avoid it becoming damp. Any on-site touching up of the painted surface should be done using an organic zinc silicate.

### 1.2 Galvanising

Galvanising is a method of corrosion protection where a sacrificial metal coating (usually zinc or zinc-aluminium alloy) is applied to the steel surface. It is particularly cost-effective for members with a high surface-to-mass ratio. (Painting is often more economical for heavy sections.) In the presence of an electrolyte – usually water with traces of dissolved salts – galvanic action occurs where one metal (the anode) corrodes at the expense of another metal (the cathode). The galvanic series is used to determine the likelihood of galvanic action occurring. The following is an example of a galvanic series relevant to the building construction industry.



Prior to galvanising, all scale, paint, oil, etc, must be removed from the surface by abrasive blasting or (more commonly) by pickling in hydrochloric or sulphuric acid. Following cleaning, the steel is rinsed and immersed in a flux solution before galvanising.

A number of design issues need to be considered before specifying galvanising, for example:





- The dimensions of the member to be galvanised check the size of the galvanising bath likely to be used and, if the member is too long or too deep to be fully galvanised by double end dipping, break it down into two or more bolted components (or specify painted protection).
- Enclosed spaces provide air vents and drainage holes to avoid rupture from expanding entrapped air.
- Stiffeners cope attached corners to prevent zinc becoming trapped as the member is lifted from the bath.
- Unsymmetrical welded sections the member will deform due to the release of locked-in stresses when heated in the galvanising bath.
- Steel having a high silicon and/or phosphorous content causes an excessive zinc build-up.
- Site welding to galvanised steel the galvanising will need to be ground off for isolated fixtures, but for beams with site welded shear studs, it may be possible to treat a strip down the middle of the top flange to prevent adherence of the zinc during galvanising. (Check with your local galvaniser.)
- Shop weld shear studs prior to galvanising for beams where steel decking is not continuous across the beam.
- The thickness of the zinc coating usually dependent on the thickness of the steel. It is usually quoted in grams per square metre, eg. over 5mm, 600 g / mm²; under 2mm, 350 g / mm². (Note that the quoted value for sheet steel products such as roofing and structural decking for composite floor slabs is the combined weight of zinc on both sides of the sheet.)

#### Painting galvanised steel

Painting of galvanised steel is not recommended due to the difficulty of achieving sufficient adherence. However, paint can increase the service life of a galvanised surface and painting is sometimes specified for aesthetic reasons. When a galvanised surface is to be painted, the surface (unless it is a weathered surface) must be first painted with a suitable etching primer.

## 1.3 Concrete encasing

Structural steel below ground is usually concrete encased. A minimum cover to the steel of 50mm is commonly used although site conditions may dictate more. A light reinforcing mesh is wrapped around the steel member to control cracking of the concrete. As the concrete must also protect the base plate of a column, the overall size of the encasing may be determined by the size of the base plate.

As concrete is a more economical material than structural steel for columns in high-rise buildings, light "erection columns" are often used to support the erected steelwork and subsequently encased in a much larger reinforced concrete column. These erection columns will form an integral part of the final column. As such, they must not be painted and splices must be detailed in such a way that voids will not be created in the concrete. (Air pockets beneath base plates, stiffeners and beam flanges will reduce the effective cross-section area of the concrete column.)

## 1.4 Protective wrapping

The major application of protective wrapping is for buried steel pipes, but this form of corrosion protection is sometimes used for light steel members that extend below ground in situations where concrete encasement is not preferred. A number of products are available, one well known one is Densotape.

## 1.5 Cathodic protection

Cathodic protection is most often used for ships and in marine works, where a high level of protection is required for structural steel and reinforcing steel in concrete. There are two methods of cathodic protection – galvanic and impressed current. Galvanic cathodic protection is used for relatively small structures and impressed current cathodic protection is used for large structures where the galvanic current is too weak to provide full protection, or for structures not submerged in an electrolyte (eg wharf decks and bridge piers).





## AUSTRALIAN STEEL INSTITUTE (ABN)/ACN (94) 000 973 839

### Composite Design Example for Multistorey Steel Framed Buildings

Copyright © 2007 by AUSTRALIAN STEEL INSTITUTE

#### Published by: AUSTRALIAN STEEL INSTITUTE

All rights reserved. This book or any part thereof must not be reproduced in any form without the written permission of Australian Steel Institute.

Note to commercial software developers: Copyright of the information contained within this publication is held by Australian Steel Institute (ASI). Written permission must be obtained from ASI for the use of any information contained herein which is subsequently used in any commercially available software package.

FIRST EDITION 2007 (LIMIT STATES)

National Library of Australia Cataloguing-in-Publication entry:

Durack, J.A. (Connell Wagner)

Kilmister, M. (Connell Wagner)

Composite Design Example for Multistorey Steel Framed Buildings

1<sup>st</sup> ed.

Bibliography.

ISBN 978-1-921476-02-0

- 1. Steel, Structural—Standards Australia.
- 2. Steel, Structural—Specifications Australia.
- 3. Composite, (Engineering)—Design and construction.
- I. Connell Wagner
- II. Australian Steel Institute.
- III. Title

Disclaimer: The information presented by the Australian Steel Institute in this publication has been prepared for general information only and does not in any way constitute recommendations or professional advice. The design examples contained in this publication have been developed for educational purposes and designed to demonstrate concepts. These materials may therefore rely on unstated assumptions or omit or simplify information. While every effort has been made and all reasonable care taken to ensure the accuracy of the information contained in this publication, this information should not be used or relied upon for any specific application without investigation and verification as to its accuracy, suitability and applicability by a competent professional person in this regard. Any reference to a proprietary product is not intended to suggest it is more or less superior to any other product but is used for demonstration purposes only. The Australian Steel Institute, its officers and employees and the authors, contributors and editors of this publication do not give any warranties or make any representations in relation to the information provided herein and to the extent permitted by law (a) will not be held liable or responsible in any way; and (b) expressly disclaim any liability or responsibility whatsoever for any loss or damage costs or expenses incurred in connection with this publication by any person, whether that person is the purchaser of this publication or not. Without limitation, this includes loss, damage, costs and expenses incurred as a result of the negligence of the authors, contributors, editors or publishers.

The information in this publication should not be relied upon as a substitute for independent due diligence, professional or legal advice and in this regards the services of a competent professional person or persons should be sought.





# **Table of contents**

Table of cor	ntents	. ii
	NPUT INFORMATION	
A1. Client a	nd Architectural Requirements	. 2
	aracteristics	
A3. Statutor	y Requirements	. 5
A4. Service	ability	. 8
	Loads	
	Is and Systems	
	Aids and Codes	
	CONCEPTUAL AND PRELIMINARY DESIGN	
•	tual and Preliminary Design	
B1.1	Consideration of alternative floor framing systems– Scheme A	
B1.2	Consideration of alternative floor framing systems– Scheme B	
B1.3	Framing system for horizontal loading – initial distribution of load	
B1.4	Alternatives for overall distribution of horizontal load to ground	
	nary Slab Design	
	Iternatives to Adopted Systems	
B3.1	Adopted floor framing arrangement	
B3.2	Adopted framing arrangement for horizontal loading	
	ve Construction Sequence and Stages	
B4.1	The importance of construction stages in composite design	
B4.1	Indicative construction sequence and construction stages	
B4.2	Adopted construction sequence for design of erection columns	
B4.3	Core construction alternatives	
B4.4	Adopted construction method for the core	
	nary Sizing of Primary and Secondary Beams	
	Requirements and Floor to Floor Height	
	ry Column Sizes and Core Wall Thickness	
	DETAILED DESIGN	
	d Design - Introduction	
	Stages and Construction Loading	
	d Load Estimation After Completion of Construction	
C3.1	Vertical loading	
C3.2	Wind loading	
C3.3		4(
	n Column Design	
C4.1	Load distribution for erection column design	
C4.2	Side Column C5 (typical of C5 to C10)	43
C4.3	End column C2 (typical of C2, C3, C12 and C13)	
C4.4	Corner column C1 (typical of columns C1, C4, C11 and C14)	44
	eams – Construction Stage 1	45
C5.1	Secondary beams Group S1(11 050, 2800) (Beams B22 – B41, B43 – 48)	
C5.2	Primary beams Group P1(9800, 5725) (Beams B1, B7 to B12, B18,	
	, B49 – 51 and B42)	46
C5.3	Primary beams Group P2(9250, 6600) (B2, B6, B13 and B17)	
	Beams – Construction Stage 3	
C6.1	Secondary beams Group S1(11 050, 2800) (Beams B22 – 41, B43 – 48)	
C6.2	Primary beams Group P1(9800, 5725) (Beams B1, B7 - B12, B18 – 21,	
	and B42)	
C6.3	Primary beams Group P2(9250, 6600) (Beams B2, B6, B13, B17)	
	eam Design for Occupancy Loading	50
C7.1	Secondary beams Group S1(11 050, 2800) (Beams B19, B21, B22 - B41,	5
R43 - R4	l9 and B51)	51





C7.2	Primary beams Group P1(9800,5725) (Beams B1, B7 to B12, B18)	58		
C7.3	Primary beams group P2(9050, 6600) (Beams B2, B6, B13, B17)	63		
C8. Assessment of Dynamic Performance of Floor System				
C8.1	Definition of the dynamic assessment process	69		
C8.2	Application of the dynamic assessment process	73		
C9 Final Slab Design				
C9.1	Slab design for the office areas			
C9.2	Slab design for the compactus areas			
C10. Longitudinal Shear Reinforcement Design				
C10.1	Introduction			
C10.2	Proprietory longitudinal shear reinforcement products			
C10.3	Secondary beams group S1, B22 typical – longitudinal shear design	84		
C10.4	Internal primary beams group P2, (B2 typical) longitudinal shear design	85		
C10.5	Primary beams P1, (B1 typical) – longitudinal shear design	87		
C10.6	Perimeter beams B19 to 21 and B49 to 51			
C11. Floor	System Design Review and Final Decisions			
C11.1	Floor design review	89		
C11.2	Final floor framing plan and deck reinforcement			
	Design of RC Columns			
	ed Design of the Core			
C13.1	Preliminary discussion and statement of limitations of this section			
C13.2	Basic modelling of the core using beam elements			
C13.3	The Space Gass Analysis Model			
C13.4	Model verification and static deflections for W <sub>s</sub>			
C13.5	Dynamic analysis for natural frequency of building			
C13.6	Interpretation and application of stress resultants from Space Gass			
C13.7	Further investigation of the core using a Strand7 finite element model			
C13.8	Review of core investigations			
	Connection Design			
C14.1	Can it be built?			
C14.2	Representative connections			
C14.3	Web side plate connection design for V* = 142 kN	108		
C14.4	Flexible end plate connection for V* = 279 kN	112		
C14.5	B2 to core web side plate connection for V* = 308 kN			
C14.6	Column splice for a load of N* = 1770 kN			
C14.7	Column base plate for a load of N* = 1770 kN			
C15. Web Penetrations				
	Final Thoughts and Disclaimers			
Appendix I Theory and discussion – composite slabs				
Appendix II Theory and discussion - composite beams				
Appendix III Dynamic assessment of the floor system				
	Appendix IV Theory and discussion steel connections			
Appendix V Corrosion and fire protection				



