Refer to the main calculations for further details relating to the implementation of the Murray Allen method for the dynamic assessment of flooring systems.

## Dynamic assessment using finite element analysis

Potentially FE modelling and analysis may be used for dynamic assessment of flooring systems, but there are problems as follows:

- Like all FE analysis great care is needed in order firstly to develop a model that is representative of reality and secondly to correctly interpret the output information
- There is a fair body of published information available relating the results of Murray Allen assessments to experimental data but little published information relating directly to FE predictions
- With the Murray Allen method, the "fiddles" involved in the assessment of W are difficult to duplicate in an FE analysis. While these fiddles may appear to be relatively ad hoc and approximate in nature, they do effectively represent an element of "wisdom" in the procedure that has been developed in order to match the results to experimental data.

Despite these problems FE analysis can be useful in visualising what is happening at a qualitative level. Figure 8 illustrates a model of a composite flooring system corresponding to that in the main design calculations. As far as possible, the model has been set up to conform with the expectations of a Murray Allen method.

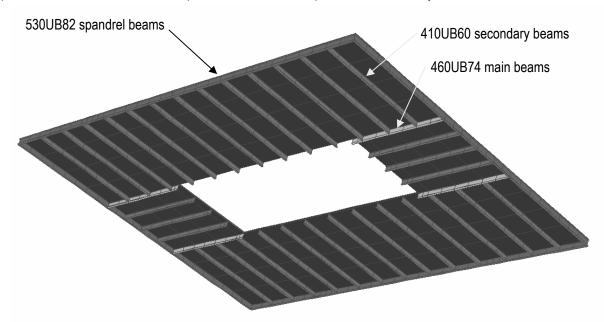


Figure 8 FE model of flooring system

Figure 9 shows the FE prediction of the first four significant natural frequency modes for this floor system. The natural frequencies predicted are generally similar to those predicted in the main calculations using equation 1, as modified to determine the combined frequency.

Note that with each of the modes there are effects similar to those illustrated in figure 6 though there may be a significant space between the different areas of dynamic movement. Modes 1 and 2 are clearly very similar. Mode 1 involves two corner regions moving in opposite directions. Mode 2 is similar but with both regions moving in the same direction. For both modes 1 and 2, it would appear that the area of slab involved in the mode is approximately twice the area of a corner panel.

Modes 3 and 4 are again similar with motion in the same direction for mode 3 and in opposite directions for mode 4. It would appear that the area of slab involved in the mode is the area of slab to the left of the core plus the area of slab to the right of the core. It would also appear that the area involved in modes 3 and 4 is significantly larger than the area involved in modes 1 and 2.





## AUSTRALIAN STEEL INSTITUTE (ABN)/ACN (94) 000 973 839

## Composite Design Example for Multistorey Steel Framed Buildings

Copyright © 2007 by AUSTRALIAN STEEL INSTITUTE

Published by: AUSTRALIAN STEEL INSTITUTE

All rights reserved. This book or any part thereof must not be reproduced in any form without the written permission of Australian Steel Institute.

Note to commercial software developers: Copyright of the information contained within this publication is held by Australian Steel Institute (ASI). Written permission must be obtained from ASI for the use of any information contained herein which is subsequently used in any commercially available software package.

FIRST EDITION 2007 (LIMIT STATES)

National Library of Australia Cataloguing-in-Publication entry:

Durack, J.A. (Connell Wagner)

Kilmister, M. (Connell Wagner)

Composite Design Example for Multistorey Steel Framed Buildings

1<sup>st</sup> ed.

Bibliography.

ISBN 978-1-921476-02-0

- 1. Steel, Structural—Standards Australia.
- 2. Steel, Structural—Specifications Australia.
- 3. Composite, (Engineering)—Design and construction.
- I. Connell Wagner
- II. Australian Steel Institute.
- III. Title

Disclaimer: The information presented by the Australian Steel Institute in this publication has been prepared for general information only and does not in any way constitute recommendations or professional advice. The design examples contained in this publication have been developed for educational purposes and designed to demonstrate concepts. These materials may therefore rely on unstated assumptions or omit or simplify information. While every effort has been made and all reasonable care taken to ensure the accuracy of the information contained in this publication, this information should not be used or relied upon for any specific application without investigation and verification as to its accuracy, suitability and applicability by a competent professional person in this regard. Any reference to a proprietary product is not intended to suggest it is more or less superior to any other product but is used for demonstration purposes only. The Australian Steel Institute, its officers and employees and the authors, contributors and editors of this publication do not give any warranties or make any representations in relation to the information provided herein and to the extent permitted by law (a) will not be held liable or responsible in any way; and (b) expressly disclaim any liability or responsibility whatsoever for any loss or damage costs or expenses incurred in connection with this publication by any person, whether that person is the purchaser of this publication or not. Without limitation, this includes loss, damage, costs and expenses incurred as a result of the negligence of the authors, contributors, editors or publishers.

The information in this publication should not be relied upon as a substitute for independent due diligence, professional or legal advice and in this regards the services of a competent professional person or persons should be sought.





## **Table of contents**

Table of cor	ntents	. ii
	NPUT INFORMATION	
A1. Client a	nd Architectural Requirements	. 2
	aracteristics	
A3. Statutor	y Requirements	. 5
A4. Service	ability	. 8
	Loads	
	Is and Systems	
	Aids and Codes	
	CONCEPTUAL AND PRELIMINARY DESIGN	
•	tual and Preliminary Design	
B1.1	Consideration of alternative floor framing systems– Scheme A	
B1.2	Consideration of alternative floor framing systems– Scheme B	
B1.3	Framing system for horizontal loading – initial distribution of load	
B1.4	Alternatives for overall distribution of horizontal load to ground	
	nary Slab Design	
	Iternatives to Adopted Systems	
B3.1	Adopted floor framing arrangement	
B3.2	Adopted framing arrangement for horizontal loading	
	ve Construction Sequence and Stages	
B4.1	The importance of construction stages in composite design	
B4.1	Indicative construction sequence and construction stages	
B4.2	Adopted construction sequence for design of erection columns	
B4.3	Core construction alternatives	
B4.4	Adopted construction method for the core	
	nary Sizing of Primary and Secondary Beams	
	Requirements and Floor to Floor Height	
	ry Column Sizes and Core Wall Thickness	
	DETAILED DESIGN	
	d Design - Introduction	
	Stages and Construction Loading	
	d Load Estimation After Completion of Construction	
C3.1	Vertical loading	
C3.2	Wind loading	
C3.3		4(
	n Column Design	
C4.1	Load distribution for erection column design	
C4.2	Side Column C5 (typical of C5 to C10)	43
C4.3	End column C2 (typical of C2, C3, C12 and C13)	
C4.4	Corner column C1 (typical of columns C1, C4, C11 and C14)	44
	eams – Construction Stage 1	45
C5.1	Secondary beams Group S1(11 050, 2800) (Beams B22 – B41, B43 – 48)	
C5.2	Primary beams Group P1(9800, 5725) (Beams B1, B7 to B12, B18,	
	, B49 – 51 and B42)	46
C5.3	Primary beams Group P2(9250, 6600) (B2, B6, B13 and B17)	
	Beams – Construction Stage 3	
C6.1	Secondary beams Group S1(11 050, 2800) (Beams B22 – 41, B43 – 48)	
C6.2	Primary beams Group P1(9800, 5725) (Beams B1, B7 - B12, B18 – 21,	
	and B42)	
C6.3	Primary beams Group P2(9250, 6600) (Beams B2, B6, B13, B17)	
	eam Design for Occupancy Loading	50
C7.1	Secondary beams Group S1(11 050, 2800) (Beams B19, B21, B22 - B41,	5
R43 - R4	l9 and B51)	51





C7.2	Primary beams Group P1(9800,5725) (Beams B1, B7 to B12, B18)	58		
C7.3	Primary beams group P2(9050, 6600) (Beams B2, B6, B13, B17)	63		
C8. Assessment of Dynamic Performance of Floor System				
C8.1	Definition of the dynamic assessment process	69		
C8.2	Application of the dynamic assessment process	73		
C9 Final Slab Design				
C9.1	Slab design for the office areas			
C9.2	Slab design for the compactus areas			
C10. Longitudinal Shear Reinforcement Design				
C10.1	Introduction			
C10.2	Proprietory longitudinal shear reinforcement products			
C10.3	Secondary beams group S1, B22 typical – longitudinal shear design	84		
C10.4	Internal primary beams group P2, (B2 typical) longitudinal shear design	85		
C10.5	Primary beams P1, (B1 typical) – longitudinal shear design	87		
C10.6	Perimeter beams B19 to 21 and B49 to 51			
C11. Floor	System Design Review and Final Decisions			
C11.1	Floor design review	89		
C11.2	Final floor framing plan and deck reinforcement			
	Design of RC Columns			
	ed Design of the Core			
C13.1	Preliminary discussion and statement of limitations of this section			
C13.2	Basic modelling of the core using beam elements			
C13.3	The Space Gass Analysis Model			
C13.4	Model verification and static deflections for W <sub>s</sub>			
C13.5	Dynamic analysis for natural frequency of building			
C13.6	Interpretation and application of stress resultants from Space Gass			
C13.7	Further investigation of the core using a Strand7 finite element model			
C13.8	Review of core investigations			
	Connection Design			
C14.1	Can it be built?			
C14.2	Representative connections			
C14.3	Web side plate connection design for V* = 142 kN	108		
C14.4	Flexible end plate connection for V* = 279 kN	112		
C14.5	B2 to core web side plate connection for V* = 308 kN			
C14.6	Column splice for a load of N* = 1770 kN			
C14.7	Column base plate for a load of N* = 1770 kN			
C15. Web Penetrations				
	Final Thoughts and Disclaimers			
Appendix I Theory and discussion – composite slabs				
Appendix II Theory and discussion - composite beams				
Appendix III Dynamic assessment of the floor system				
	Appendix IV Theory and discussion steel connections			
Appendix V Corrosion and fire protection				



