## PART B BOLTED/WELDED COVER PLATE SPLICE

#### B15 Design capacity tables

The following DESIGN CAPACITY TABLES are provided, derived using DESIGN CHECK NOS 1 to 8 inclusive.

- Table B7 Design moment capacity of bolted/welded single cover plate splice Universal beam sections < 400 deep, M20 bolts, 6 fillets to flange plates, 5 fillets to web plates
- Table B8 Design moment capacity of bolted/welded single cover plate splice
  Universal beam sections > 400 deep, M24 bolts, 8 or 6 fillets to flange plates,
  5 fillets to web plates
- Table B9 Design moment capacity of bolted/welded three cover plate splice Universal column sections, M24 bolts, 6/8 fillets to flange plates and 6 fillets to web plates
- Table B10 Design moment capacity of bolted/welded three cover plate splice 700WB/800WB welded beam sections, M24 bolts, 6/8 fillets to flange plates and 5 fillets to web plates
- Table B11 Design moment capacity of bolted/welded three cover plate splice 900WB/1000WB welded beam sections, M24 bolts, 6/8 fillets to flange plates and 6 fillets to web plates



#### TABLE B7

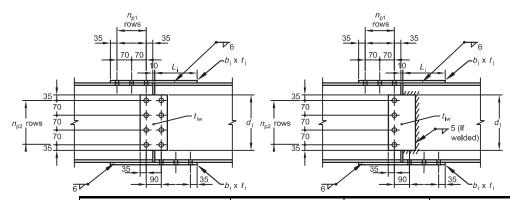
## DESIGN MOMENT CAPACITY OF BOLTED/WELDED SINGLE COVER PLATE SPLICE UNIVERSAL BEAM SECTIONS < 400 DEEP M20 BOLTS, 6 FILLETS TO FLANGE PLATES, 5 FILLETS TO WEB PLATES

Section: Grade 300 Bolts: 8.8/TB Category, 70 gauge in flange

Cover plates: Grade 250 plate Axial force: See Note 2

Shear force: Max [0.15 × design shear cap; 40 kN] Welds: SP category E40XX/W50X electrodes

limited to 0.60 × design shear cap



			Flange—1 Plate (Note 3)				Web—2 Plates			CASE 1		CASE 2, <i>N</i> *≠0		
										Max V*	N *=0	Max V*	Max N*	Refer Note 1
Section, Grade 300	φMs	φM <sub>S</sub> '	<i>b</i> i	ti	n <sub>p1</sub>	<i>L</i> i	di	t <sub>iw</sub>	n <sub>p2</sub>	(plus or minus)	φ <i>M</i> conn	(plus or minus)	(Tens or Comp)	φ <i>M</i> conn
Grade 300	kNm	kNm								kN	kNm	kN	kN	kNm
360UB56.7	273	245	150	20	4	280	280	6	4	297	193	297	98.0	183
			150	16	3	210	280	6	4	297	184	297	98.0	173
			150	12	3	210	280	6	4	75.0	163	74.4	98.0	150
360UB50.7	242	217	150	16	3	210	280	6	4	269	169	269	87.5	159
			150	12	2	140	280	6	4	67.4	143	67.4	87.5	132
			150	10	2	140	280	6	4	93.7	136	91.4	87.5	125
360UB44.7	222	198	150	16	3	210	280	6	4	251	141	251	82.5	133
			150	12	3	210	280	6	4	251	136	251	82.5	128
			150	10	2	140	280	6	4	63.0	123	63.0	82.5	112
310UB46.2	197	174	150	16	3	210	210	6	3	213	143	213	80.0	135
			150	12	3	210	210	6	3	213	117	213	80.0	109
			150	10	2	140	210	6	3	213	98.1	213	80.0	90.1
310UB40.4	182	161	150	16	3	210	210	6	3	191	122	191	75.0	114
			150	12	3	210	210	6	3	191	117	191	75.0	109
			150	10	2	140	210	6	3	191	97.7	191	75.0	90.3
310UB32.0	134	117	130	12	2	140	210	6	3	169	82.0	169	59.0	76.7
			130	10	2	140	210	6	3	169	78.2	169	59.0	72.9
			130	8	2	140	210	6	3	169	62.6	169	59.0	57.3
250UB37.3	140	121	130	16	2	140	210	6	3	42.5	94.8	42.5	68.5	86.8
			130	12	2	140	210	6	3	53.7	90.8	49.6	68.5	84.2
_			130	10	2	140	210	6	3	96.0	75.6	91.9	68.5	69.1
250UB31.4	114	99.1	130	10	2	140	210	6	3	40.0	75.6	40.0	57.5	69.2
			130	8	2	140	210	6	3	86.1	61.8	82.5	57.5	56.5

#### NOTES:



<sup>1</sup>  $\phi M_{conn}$  = design moment capacity of connection

 $<sup>\</sup>phi M_s$  = design section moment capacity of full section

φMs' = design section moment capacity of section with holes in one flange

<sup>2</sup> Case 1 applies to splices with no axial force ( $N^*=0$ )

Case 2 applies to splices where axial force N\* does not exceed the value tabulated (approx 5% of design section capacity)

<sup>3 6</sup> mm plate not used for flange cover plates due to concerns about plate buckling

# Design Guide 13 Splice connections

by

T.J. Hogan

contributing author

N. van der Kreek

first edition—2009



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### Design Guide 13 Splice connections

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