PART B BOLTED/WELDED COVER PLATE SPLICE

B15 Design capacity tables

The following DESIGN CAPACITY TABLES are provided, derived using DESIGN CHECK NOS 1 to 8 inclusive.

- Table B7 Design moment capacity of bolted/welded single cover plate splice Universal beam sections < 400 deep, M20 bolts, 6 fillets to flange plates, 5 fillets to web plates
- Table B8 Design moment capacity of bolted/welded single cover plate splice
 Universal beam sections > 400 deep, M24 bolts, 8 or 6 fillets to flange plates,
 5 fillets to web plates
- Table B9 Design moment capacity of bolted/welded three cover plate splice Universal column sections, M24 bolts, 6/8 fillets to flange plates and 6 fillets to web plates
- Table B10 Design moment capacity of bolted/welded three cover plate splice 700WB/800WB welded beam sections, M24 bolts, 6/8 fillets to flange plates and 5 fillets to web plates
- Table B11 Design moment capacity of bolted/welded three cover plate splice 900WB/1000WB welded beam sections, M24 bolts, 6/8 fillets to flange plates and 6 fillets to web plates



TABLE B7

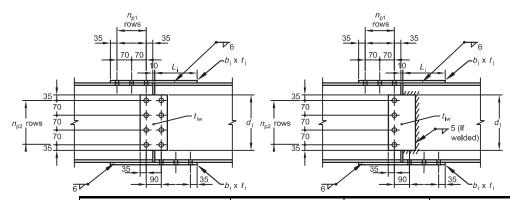
DESIGN MOMENT CAPACITY OF BOLTED/WELDED SINGLE COVER PLATE SPLICE UNIVERSAL BEAM SECTIONS < 400 DEEP M20 BOLTS, 6 FILLETS TO FLANGE PLATES, 5 FILLETS TO WEB PLATES

Section: Grade 300 Bolts: 8.8/TB Category, 70 gauge in flange

Cover plates: Grade 250 plate Axial force: See Note 2

Shear force: Max [0.15 × design shear cap; 40 kN] Welds: SP category E40XX/W50X electrodes

limited to 0.60 × design shear cap



			Flange—1 Plate (Note 3)				Web—2 Plates			CASE 1		CASE 2, <i>N</i> *≠0		
										Max V*	N *=0	Max V*	Max N*	Refer Note 1
Section, Grade 300	φMs	φM _S '	<i>b</i> i	<i>t</i> i	n _{p1}	<i>L</i> i	di	t _{iw}	n _{p2}	(plus or minus)	φ <i>M</i> conn	(plus or minus)	(Tens or Comp)	φ <i>M</i> conn
Grade 300	kNm	kNm								kN	kNm	kN	kN	kNm
360UB56.7	273	245	150	20	4	280	280	6	4	297	193	297	98.0	183
			150	16	3	210	280	6	4	297	184	297	98.0	173
			150	12	3	210	280	6	4	75.0	163	74.4	98.0	150
360UB50.7	242	217	150	16	3	210	280	6	4	269	169	269	87.5	159
			150	12	2	140	280	6	4	67.4	143	67.4	87.5	132
			150	10	2	140	280	6	4	93.7	136	91.4	87.5	125
360UB44.7	222	198	150	16	3	210	280	6	4	251	141	251	82.5	133
			150	12	3	210	280	6	4	251	136	251	82.5	128
			150	10	2	140	280	6	4	63.0	123	63.0	82.5	112
310UB46.2	197	174	150	16	3	210	210	6	3	213	143	213	80.0	135
			150	12	3	210	210	6	3	213	117	213	80.0	109
			150	10	2	140	210	6	3	213	98.1	213	80.0	90.1
310UB40.4	182	161	150	16	3	210	210	6	3	191	122	191	75.0	114
			150	12	3	210	210	6	3	191	117	191	75.0	109
			150	10	2	140	210	6	3	191	97.7	191	75.0	90.3
310UB32.0	134	117	130	12	2	140	210	6	3	169	82.0	169	59.0	76.7
			130	10	2	140	210	6	3	169	78.2	169	59.0	72.9
			130	8	2	140	210	6	3	169	62.6	169	59.0	57.3
250UB37.3	140	121	130	16	2	140	210	6	3	42.5	94.8	42.5	68.5	86.8
			130	12	2	140	210	6	3	53.7	90.8	49.6	68.5	84.2
_			130	10	2	140	210	6	3	96.0	75.6	91.9	68.5	69.1
250UB31.4	114	99.1	130	10	2	140	210	6	3	40.0	75.6	40.0	57.5	69.2
			130	8	2	140	210	6	3	86.1	61.8	82.5	57.5	56.5

NOTES:



¹ ϕM_{conn} = design moment capacity of connection

 $[\]phi M_s$ = design section moment capacity of full section

φMs' = design section moment capacity of section with holes in one flange

² Case 1 applies to splices with no axial force ($N^*=0$)

Case 2 applies to splices where axial force N* does not exceed the value tabulated (approx 5% of design section capacity)

^{3 6} mm plate not used for flange cover plates due to concerns about plate buckling

Design Guide 13 Splice connections

by

T.J. Hogan

contributing author

N. van der Kreek

first edition—2009



AUSTRALIAN STEEL INSTITUTE (ABN)/ACN (94) 000 973 839

Design Guide 13 Splice connections

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Published by: AUSTRALIAN STEEL INSTITUTE

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FIRST EDITION 2009 (LIMIT STATES)

National Library of Australia Cataloguing-in-Publication entry:

Hogan, T.J.

Design Guide 13: Splice connections

1st ed.

Bibliography.

ISBN 978 1 921476 16 7 (pbk.). ISBN 978 1 921476 17 4 (pdf.).

1. Steel, Structural—Standards – Australia.

2. Steel, Structural—Specifications – Australia.

3. Joints, (Engineering)—Design and construction.

I. van der Kreek, N.

II. Australian Steel Institute.

III. Title

(Series: Structural steel connection series).

This publication originated as part of Design of structural connections

First edition 1978 Second edition 1981 Third edition 1988 Fourth edition 1994

Also in this series:

Design capacity tables for structural steel. Volume 3: Simple connections—Open sections

Handbook 1: Design of structural steel connections

Design Guide 1: Bolting in structural steel connections

Design Guide 2: Welding in structural steel connections

Design Guide 3: Web side plate connections

Design Guide 4: Flexible end plate connections

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Design capacity tables for structural steel. Volume 4: Rigid connections—Open sections

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