Tubular Design Guide 22: Bolted bracing cleats

by

P.W. Key and A.A. Syam

first edition - 2014



AUSTRALIAN STEEL INSTITUTE (ABN)/ACN (94) 000 973 839

Tubular Design Guide 22 Bolted bracing cleats

Copyright © 2014 by AUSTRALIAN STEEL INSTITUTE

Published by: AUSTRALIAN STEEL INSTITUTE

All rights reserved. This book or any part thereof must not be reproduced in any form without the written permission of Australian Steel Institute.

Note to commercial software developers: Copyright of the information contained within this publication is held by Australian Steel Institute (ASI). Written permission must be obtained from ASI for the use of any information contained herein which is subsequently used in any commercially available software package.

FIRST EDITION 2014 (LIMIT STATES)

National Library of Australia Cataloguing-in-Publication entry:

Key, Peter W. Tubular Design Guide 22: Bolted bracing cleats / Peter W. Key, Arun A. Syam.

ISBN 978 1 921476 31 0 (pbk.). Series: Structural steel tubular connection series. Includes bibliographical references. Steel, Structural—Standards - Australia. Structural engineering. Syam, Arun A. Australian Steel Institute. 624.1821021894

Also in this series: Tubular Design Guide 20: Background and design basis Tubular Design Guide 21: Bolted bracing connections Tubular Design Guide 23: Plate fitments Tubular Design Guide 24: Bolted planar connections Tubular Design Guide 25: Fully welded – Simple planar connections Tubular Design Guide 26: Fully welded – Gap planar connections Tubular Design Guide 27: Fully welded – Overlap planar connections

Disclaimer: The information presented by the Australian Steel Institute in this publication has been prepared for general information only and does not in any way constitute recommendations or professional advice. While every effort has been made and all reasonable care taken to ensure the accuracy of the information contained in this publication, this information should not be used or relied upon for any specific application without investigation and verification as to its accuracy, suitability and applicability by a competent professional person in this regard. The Australian Steel Institute, its officers and employees and the authors and editors of this publication do not give any warranties or make any representations in relation to the information provided herein and to the extent permitted by law (a) will not be held liable or responsible in any way; and (b) expressly disclaim any liability or responsibility for any loss or damage costs or expenses incurred in connection with this publication by any person, whether that person is the purchaser of this publication or not. Without limitation, this includes loss, damage, costs and expenses incurred as a result of the negligence of the authors, editors or publishers.

The information in this publication should not be relied upon as a substitute for independent due diligence, professional or legal advice and in this regards the services of a competent professional person or persons should be sought.



ii 👔

Lis Pro Ab	efac	Figures Tables e the Aut wledger	hors	v vi vii viii ix
1	1.1	Backgr	OF DESIGN GUIDES round ed connections	1-1 1-1 1-2
2	2.1	Techni 2.1.1 2.1.2 2.1.3		2-1 2-1 2-1 2-1 2-4
	2.2	2.2.1 2.2.2	ties of connection elements General Component material yield	2-5 2-5
		2.2.3 2.2.4	stress SSHS material properties Design yield stress for	2-5 2-6
		2.2.5 2.2.6	Australian produced SSHS Design capacity of welds Design capacity of bolts	2-7 2-7 2-8
	2.3	Refere	nces for Sections 1 and 2	2-9
3				3-1
			otion of connection	3-1 3-3
			l detailing of connection	
				3-4
	3.4	-	ound information	3-6
		3.4.1	Recent research	3-6
	35	3.4.2 Compli	Relevant international codes and specifications ance with AS 4100	3-7
	0.0	-	ements	3-8
	36		of design model	3-10
			ation of design actions	3-12
		Recom	imended design model— ction notation	3-13
	3.9	Recom	imended design model— of validity	3-14
	3.10	Recom	nmended design model—	
	3.11		ary of design checks nmended design model—	3-15
		Gener	al	3-16
		3.11.1	DESIGN CHECK NO. 1-	
		3.11.2	Detailing requirements DESIGN CHECK NO. 2—	3-16
		3.11.3		3-17
			Design capacity of bolts in shear and ply bearing	3-18

	Page
3.11.4 DESIGN CHECK NO. 4-	
Design capacity due to end	
plate tearout in bracing cleat	3-19
3.11.5 DESIGN CHECK NO. 5-	
Design capacity of bracing cl	
in block shear	3-20
3.11.6 DESIGN CHECK NO. 6-	
Design capacity of bracing cleat in axial tension	2 22
3.11.7 DESIGN CHECK NO. 7—	3-22
Design capacity of welds to	
SSHS member	3-23
3.11.8 DESIGN CHECK NO. 8—	0 20
Design capacity of SSHS	
member locally—Face shear	3-24
3.11.9 DESIGN CHECK NO. 9-	
Design capacity of SSHS	
member locally—Face	
yielding	3-25
3.11.10 DESIGN CHECK NO. 10-	
Design capacity of bolts in	
shear and ply bearing	3-28
3.11.11 DESIGN CHECK NO. 11-	
Design capacity of weld to	0.00
SSHS member	3-29
3.11.12 DESIGN CHECK NO. 12— Design capacity of SSHS	
member locally—	
Face shear	3-30
3.11.13 DESIGN CHECK NO. 13—	0.00
Design capacity of SSHS	
member locally—	
Face yielding	3-31
3.11.14 DESIGN CHECK NO. 14-	
Design capacity of bracing	
cleat under eccentric	
compression force	3-34
3.12 Design example	3-36
3.13 Design capacity tables	3-45
3.14 References	3-58
GUSSET PLATE	4-1
4.1 Description of connection	4-1 4-1
4.2 Description of connection	4-3
4.3 Detailing considerations	4-4
4.4 Background information	4-6
4.4.1 Recent research	4-6
4.4.2 Relevant international	-
codes and specifications	4-11
4.5 Compliance with AS 4100	
requirements	4-12
4.6 Basis of design model	4-13
4.7 Calculation of design actions	4-15



4

iii iii

4.8		mended design model—	
10		ction notation mended design model—	4-17
4.3		of validity	4-18
4.10		mended design model—	
		ary of design checks	4-20
4 1 1		mended design model—	•
	Genera	-	4-21
		DESIGN CHECK NO. 1—	
		Detailing requirements	4-21
	4.11.2	DESIGN CHECK NO. 2—	
	7.11.2	Validity limits and minimum	
		design actions	4-22
	4.11.3	DESIGN CHECK NO. 3—	4-22
	4.11.3		
		Design capacity of bolts	4 00
		in shear and bearing	4-23
	4.11.4	DESIGN CHECK NO. 4-	
		Design capacity due to end	
		plate tearout in gusset plate	4-24
	4.11.5	DESIGN CHECK NO. 5-	
		Design capacity of gusset	
		plate in block shear	4-25
	4.11.6	DESIGN CHECK NO. 6-	
		Design capacity of gusset	
		plate in axial tension	4-29
	4.11.7	DESIGN CHECK NO. 7-	
		Design capacity of bolts in	
		shear and ply bearing	4-30
	4.11.8	DESIGN CHECK NO. 8-	
		Design capacity of gusset	
		under eccentric compression	
		force	4-31
	4.11.9	DESIGN CHECK NO. 9—	101
	4.11.5	Design capacity of gusset	
		plate—critical section yield	4-34
	1 1 1 10	DESIGN CHECK NO. 10—	4-04
	4.11.10		
		Design capacity of welds to	4.05
		SSHS supporting member	4-35
	4.11.11	DESIGN CHECK NO. 11-	
		Design capacity of SSHS	
		member locally—Face shear	4-37
	4.11.12	DESIGN CHECK NO. 12-	
		Design capacity of SSHS	
		member locally—Chord face	
		yielding	4-38

Page

	4.11.13 DESIGN CHECK NO. Design capacity of SSI member locally—	
	Punching shear	4-41
	4.12 Design example	4-42
	4.13 Design capacity tables	4-48
	4.14 References	4-49
5	CLEAT PLATE ASSEMBLY	5-1
	5.1 Description of connection	5-1
	5.2 Typical detailing of connection	
	5.3 Detailing considerations	5-5
	5.4 Background information	5-7
	5.4.1 Recent research	5-7
	5.4.2 Recommendations	5-9
	5.5 Compliance with AS 4100	
	requirements	5-12
	5.6 Basis of design model	5-14
	5.7 Calculation of design actions	5-15
	5.8 Recommended design model	
	Connection notation	5-16
	5.9 Recommended design model	
	Limits of validity	5-17
	5.10 Recommended design mode	I—
	Summary of design checks	5-18
	5.11 Recommended design mode	I—
	General	5-19
	5.11.1 DESIGN CHECK NO.	1—
	Detailing requirements	5-19
	5.11.2 DESIGN CHECK NO.	2—
	Validity limits	5-20
	5.11.3 DESIGN CHECK NO.	3—
	Design capacity of clea	at plate
	assembly under eccen	•
	compression force	5-21
	5.12 Design example	5-24
	5.13 Design capacity tables	5-28
	5.14 References	5-35
6	NOTATION AND ABBREVIATION	IS 6-1
AF	PPENDICES	
	A—Limcon design example outpu B—ASI Design Guide 22	t 7-1
	Comment form	8-1



iv iv

Page

LIST OF FIGURES

Page

		U
Figure 1.1	Connections included in this	4.0
Figure 2.1	Design Guide Definition of element width	1-2
	for RHS flanges	2-3
Figure 3.1	Configuration of bracing clea	
Figure 3.2	Bracing cleat slotted through	
	SSHS supporting member	3-2
Figure 3.3	Single and double line of	
	bolts to single bracing cleat	
	connected to face of support	
	CHS, SHS and RHS membe	rs 3-3
Figure 3.4	Single and double line of	
	bolts to single bracing cleat	
	slotted through support: CHS	
Figure 2 F	SHS and RHS members	3-3
Figure 3.5	Edge distances for bolt holes Eccentricities at cleats	s 3-5 3-8
Figure 3.6 Figure 3.7	Design actions at connection	
Figure 3.8	Notation for bracing cleat de	
rigule 5.0	model	3-13
Figure 3.9	Detailing of bolt holes in	5-15
riguie 0.0	bracing cleat	3-19
Figure 3.10) End plate block shear areas	0 10
· · · go. o · · · ·	in bracing cleat component	3-21
Figure 3.11	Effective length of cleat	
9	assembly	3-35
Figure 3.12	2 Design example	
-	configuration	3-36
Figure 3.13	B Assumed cleat assembly	
	dimensions	3-44
Figure 4.1		
	gusset plates	4-1
Figure 4.2		
	SSHS member	4-2
Figure 4.3	Single and double line of	
	bolts to single bracing cleat	
	connected to gusset plate:	ro 1 2
Figuro 4.4	CHS, SHS and RHS membe Edge distances for bolt holes	
Figure 4.4 Figure 4.5	Examples of critical sections	5 4-0
rigule 4.5	for design model checks on	
	gusset plates	4-7
Figure 4.6	Whitmore effective width	• •
- gare rie	concept	4-8
Figure 4.7	Assignment of Whitmore	
U U		
	sections on gusset plate	
Figure 4.8	sections on gusset plate with 2 bolt connections	4-8
		4-8
	with 2 bolt connections Connection forces with and without noding eccentricity	4-8 4-9
Figure 4.9	with 2 bolt connections Connection forces with and without noding eccentricity Generic gusset plate	4-9
-	with 2 bolt connections Connection forces with and without noding eccentricity Generic gusset plate configurations	
-	with 2 bolt connections Connection forces with and without noding eccentricity Generic gusset plate configurations Eccentricity at cleat to	4-9 4-10
-	with 2 bolt connections Connection forces with and without noding eccentricity Generic gusset plate configurations	4-9

	Page
Figure 4.11 Connection forces with an without noding eccentricity	/ 4-15
Figure 4.12 Design actions for multiple brace members connected to single gusset plate	
Figure 4.13 Notation for gusset plate connection	4-17
Figure 4.14 Definition of gusset plate edge buckling parameters	4-19
Figure 4.15 Minimum edge distances for bolt holes in gusset pla	
Figure 4.16 End plate block shear area	as
in connection component Figure 4.17 Corner tearout failure mod	
in gusset plates Figure 4.18 Definition of Whitmore section for bracing	4-27
connection to gusset plate Figure 4.19 Effective length of cleat	4-30
plate assembly	4-33
Figure 4.20 Assumed critical section for gusset plate	4-34
Figure 4.21 Gusset plate weld design actions	4-36
Figure 4.22 Design example configuration	4-42
Figure 4.23 Derived geometric parameters for design	
example Figure 5.1 Typical configurations of	4-43
cleat plate assemblies Figure 5.2 Typical forked cleat plate	5-1
assembly	5-2
Figure 5.3 Generic cleat plate assem and modelling of cleat-to-	bly
gusset connection Figure 5.4 Single and double line of	5-3
bolts to cleat assembly	5-4
Figure 5.5 Edge distances for bolt ho	les 5-6
Figure 5.6 Gusset plate connection configurations	5-8
Figure 5.7 Eccentricities of single cleat plate assembly	5-12
Figure 5.8 Eccentricity of forked cleat plate assembly	5-13
Figure 5.9 Design actions considered	l
at the connection Figure 5.10 Notation for cleat plate	5-15
assembly design model	5-16
Figure 5.11 Effective length of cleat assembly	5-22



v v

	Page		Page
Figure 5.12 Details and behaviour of forked cleat plate model	5-23	Figure 5.14 Cleat assembly dimensions Figure 5.15 Standardised cleat plate	5-27
Figure 5.13 Design example	5-25	assembly	5-28
configuration	5-24		

LIST OF TABLES

Page

Table 2.1 Rationalised validity limits for SSHS bracing connections	2-2
Table 2.2 Section slenderness limits to	2-2
AS 4100 and Eurocode 3	2-3
Table 2.3 Section slenderness limits to	
Eurocode 3 for design grades	
(f_y) 250, 350 and 450 MPa	2-4
Table 2.4 Minimum plate material	
properties to AS/NZS 3678	2-5
Table 2.5 Strength of flat bars to	
AS/NZS 3679.1 Grade 300	2-6
Table 2.6 Minimum SSHS material	
properties to AS/NZS 1163	2-6
Table 2.7 Design yield stress for SSHS	
related failure modes for SSHS	
to AS/NZS 1163	2-7
Table 2.8 Strength limit state design	
capacities of equal leg fillet welds	
per unit length Category SP, $\phi = 0.8$,	
$k_{\rm r}$ = 1.0, material thickness ≥3 mm	2-7
Table 2.9 Strength limit state high strength	
structural bolts 8.8/S, 8.8/TB, 8.8/TF	
bolting categories ($f_{uf} = 830 \text{ MPa}$)	2-8
Table 3.1 Rationalised validity limits for	
bracing cleat connection	3-14
Table 3.13.1(a) Bracing cleat axial design	
capacity – 'small' CHS –	
Grade 250	3-46
Table 3.13.1(b) Bracing cleat axial design	
capacity – 'large' CHS –	o 47
Grade 250	3-47
Table 3.13.2(a) Bracing cleat axial design	
capacity – 'small' CHS –	0.40
Grade 350	3-48
Table 3.13.2(b) Bracing cleat axial design capacity – 'large' CHS –	
Grade 350	3-49
Grade 350	3-49

	Page
Table 3.13.3(a) Bracing cleat axial design capacity – 'small' SHS –	0.50
Grade 350 Table 3.13.3(b) Bracing cleat axial design	3-50
capacity – 'large' SHS – Grade 350	3-51
Table 3.13.4(a) Bracing cleat axial design capacity – 'small' SHS –	
Grade 450 Table 3.13.4(b) Bracing cleat axial design	3-52
capacity – 'large' SHS – Grade 450	3-53
Table 3.13.5(a) Bracing cleat axial design capacity – 'small' RHS –	
Grade 350 Table 3.13.5(b) Bracing cleat axial design	3-54
capacity – 'large' RHS – Grade 350	3-55
Table 3.13.6(a) Bracing cleat axial design capacity – 'small' RHS –	0.00
Grade 450 Table 3.13.6(b) Bracing cleat axial design	3-56
capacity - 'large' RHS -	0.57
Grade 450 Table 4.1 Rationalised validity limits for	3-57
gusset plate connection Table 4.2 Definition of gusset plate edge	4-18
buckling parameters Table 4.3 Gusset plate slenderness values	4-19 4-22
Table 5.1 Rationalised validity limits	
for cleat plate assembly Table 5.13.1(a) Cleat plate assembly – 'sma	5-17 II' –
axial compression capacity ϕN_{conn}	5-29
Table 5.13.1(b) Cleat plate assembly – 'large	
axial compression capacity ϕN_{conn}	5-30
Table 5.13.2(a) Cleat plate assembly – 'sma	
axial tension capacity ϕN_{conn}	5-32
Table 5.13.2(b) Cleat plate assembly – 'larg	
axial tension capacity ϕN_{conn}	5-33



PREFACE

This new series of connection publications by the Australian Steel Institute (ASI) covering design capacity tables, theory and design of individual structural steel hollow section ('tubular') member connections will be known as the Structural Steel Tubular Connection Series: 1st edition 2013 ('Tubular Connection Series'). The Tubular Connection Series details the method of design and provides design capacity tables and detailing parameters for a range of tubular connections commonly used in Australia. Connections have a major engineering and economic importance in steel structures influencing design, detailing, fabrication and erection costs. Standardisation of design approach integrated with industry detailing is the key to minimum costs at each stage. The Tubular Connection Series is written in the same format as and extends the range of the existing 'Structural Steel Connection Series Parts 1 and 2' published by ASI commencing 2007. Each book in the new Tubular Connection Series is numbered as a continuation from the existing series and hence this current book is referred to as 'Tubular Design Guide 22' (TDG22). The Tubular Connection Series replaces and enhances an AISC publication released in 1996 and titled 'Design of structural steel hollow section connections' (often referred to as the 'Blue Book'). With significant international research undertaken in the interim period and new and refined design models available, together with improvements in the performance of Australian produced structural steel hollow sections (SSHS), the time was appropriate to revise and update the Blue Book.

Tubular Design Guide 22 brings together a number of design models for the cleats associated with connections to bracing members in frames that are usually subjected to predominantly axial tension and/or compression and are pin-ended. The format and intent of the technical components of TDG22 is to provide sufficient technical basis to allow TDG22 to be a self-standing document, but at the same time, where substantive background technical basis is required, the reader may refer back to both Tubular Design Guide 20 (TDG20) and Handbook 1 of the existing Structural Steel Connection Series.

This has been achieved through extensive local and international literature reviews using ASI's close association with like organisations and searching the wealth of material contained in the ASI Library (the largest steel design library in the Southern Hemisphere). This process consolidated industry best practice, references and research papers. TDG22, in conjunction with TDG20 and Handbook 1, formulates the design models and procedures for the assessment of bolts, bolt groups, welds, weld groups, connection components and supporting members associated with the end connections to SSHS bracing members.

Following on from the existing Structural Steel Connection Series, the new Tubular Connection Series format, with separate design guides for individual connection types or groupings related to similar functions, is intended to facilitate addition to, or revision of, connection model theory using relevant new local or international research. Connection models developed follow a stylised page format with a numbered DESIGN CHECK procedure to simplify connection capacity assessment. Combined with a worked example and accompanying design capacity tables, each connection model provides a self-standing solution for the design engineer.

Engineering Systems has worked closely with the ASI to further develop their existing Limcon software as the companion program for this new Tubular Connection Series. The latest version of Limcon (V3.6) fully implements the new connection design models and was employed in checking the design tables. The Limcon output for one or more of the worked examples is included in an appendix to each design guide for each connection design type. The program is an efficient tool covering the full range of structural connections, including those beyond the scope of the design capacity tables provided in the Tubular Connection Series.

The existing Structural Steel Connection Series included comment/feedback forms. In the current series, these are replaced by a recently developed web based eForum facility. Every publication, seminar and talk that ASI sponsors has or will have a corresponding thread on the ASI eForum. Users are encouraged to log into the eForum and provide feedback on this current series. The eForum is located off our website at http://steel.org.au/forum/

P.W. Key A.A. Syam



vii Vii